Braja M. Das | Khaled Sobhan

PRINCIPLES OF GEOTECHNICAL ENGINEERING

Ninth Edition

Principles of Geotechnical Engineering

Ninth Edition

Copyright 2018 Cengage Learning. All Rights Reserved. May not be copied, scanned, or duplicated, in whole or in part. WCN 02-200-203

Copyright 2018 Cengage Learning. All Rights Reserved. May not be copied, scanned, or duplicated, in whole or in part. WCN 02-200-203

Principles of Geotechnical Engineering

Ninth Edition

BRAJA M. DAS, Dean Emeritus

California State University, Sacramento

KHALED SOBHAN, Professor

Florida Atlantic University

Australia ● Brazil ● Mexico ● Singapore ● United Kingdom ● United States

CENGAGE Learning[®]

Principles of Geotechnical Engineering, Ninth Edition

Authors: Braja M. Das and Khaled Sobhan

Product Director, Global Engineering: Timothy L. Anderson

Senior Content Developer: Mona Zeftel

Product Assistant: Alexander Sham

Marketing Manager: Kristin Stine

Director, Higher Education Production: Sharon L. Smith

Content Project Manager: Jana Lewis

Production Service: RPK Editorial Services, Inc.

Copyeditor: Lori Martinsek

Proofreader: Harlan James

Indexer: Braja M. Das

Compositor: MPS Limited

Senior Art Director: Michelle Kunkler

Cover and Internal Designer: Harasymczuk Design

Cover Image: Felipe Gabaldon, Getty Images

Intellectual Property

Analyst: Christine Myaskovsky

Project Manager: Sarah Shainwald

Text and Image Permissions Researcher: Kristiina Paul

Manufacturing Planner: Doug Wilke

© 2018, 2014 Cengage Learning®

ALL RIGHTS RESERVED. No part of this work covered by the copyright herein may be reproduced or distributed in any form or by any means, except as permitted by U.S. copyright law, without the prior written permission of the copyright owner.

For product information and technology assistance, contact us at **Cengage Learning Customer & Sales Support, 1-800-354-9706**.

For permission to use material from this text or product, submit all requests online at **www.cengage.com/permissions**. Further permissions questions can be emailed to **permissionrequest@cengage.com**.

Library of Congress Control Number: 2016942336

ISBN: 978-1-305-97093-9

Cengage Learning

20 Channel Center Street Boston, MA 02210 USA

Cengage Learning is a leading provider of customized learning solutions with employees residing in nearly 40 different countries and sales in more than 125 countries around the world. Find your local representative at **www.cengage.com**.

Cengage Learning products are represented in Canada by Nelson Education Ltd.

To learn more about Cengage Learning Solutions, visit **www.cengage.com/engineering**.

Purchase any of our products at your local college store or at our preferred online store **www.cengagebrain.com**.

Unless otherwise noted, all items © Cengage Learning.

Printed in the United States of America Print Number: 01 Print Year: 2016 *To Elizabeth Madison, Armaan and Shaiza*

Copyright 2018 Cengage Learning. All Rights Reserved. May not be copied, scanned, or duplicated, in whole or in part. WCN 02-200-203

PREFACE

Principles of Geotechnical Engineering is intended for use as a text for the introductory course in geotechnical engineering taken by practically all civil engineering students, as well as for use as a reference book for practicing engineers. The book has been revised in 1990, 1994, 1998, 2002, 2006, and 2010. The eighth edition was published in 2014 with coauthor, Khaled Sobhan of Florida Atlantic University. As in the previous editions of the book, this new edition offers a valuable overview of soil properties and mechanics, together with coverage of field practices and basic engineering procedures. It is not the intent of this book to conform to any design codes. The authors appreciate the overwhelming adoptions of this text in various classrooms and are gratified that it has become the market-leading textbook for the course.

New to the Ninth Edition

- This edition includes many new example problems as well as revisions to existing problems. This book now offers more than 185 example problems to ensure understanding. The authors have also added to and updated the book's end-of-chapter problems throughout.
- In Chapter 1 on "Geotechnical Engineering: A Historical Perspective," the list of ISSMGE (International Society for Soil Mechanics and Geotechnical Engineering) technical committees (as of 2013) has been updated. A list of some important geotechnical engineering journals now in publication has been added.
- Chapter 2 on "Origin of Soil and Grain Size" has a more detailed discussion on U.S. sieve sizes. British and Australian standard sieve sizes have also been added.
- Chapter 3 on "Weight-Volume Relationships" now offers an expanded discussion on angularity and the maximum and minimum void ratios of granular soils.
- Students now learn more about the fall cone test used to determine the liquid limit in Chapter 4, which covers "Plasticity and Structure of Soil."
- In Chapter 6 on "Soil Compaction," a newly-developed empirical correlation for maximum dry density and optimum moisture content has been added.
- In Chapter 7 on "Permeability," sections on permeability tests in auger holes, hydraulic conductivity of compacted clay soils, and moisture content-unit weight criteria for clay liner construction have been added.
- Pavlovsky's solution for seepage through an earth dam has been added to Chapter 8 on "Seepage."
- Chapter 10 on "Stresses in a Soil Mass," has new sections on:
	- • Vertical stress caused by a horizontal strip load,
	- • Westergaard's solution for vertical stress due to a point load, and
	- • Stress distribution for Westergaard material.
- An improved relationship for elastic settlement estimation has been incorporated into Chapter 11 on "Compressibility of Soil." This chapter also has a new section on construction time correction (for ramp loading) of consolidation settlement.
- Chapter 12 on "Shear Strength of Soil" now includes some recently-published correlations between drained angle of friction and plasticity index of clayey soil. Additional content has been included on the relationship between undrained shear strength of remolded clay with liquidity index.
- The generalized case for Rankine active and passive pressure (granular backfill) now appears in Chapter 13 on "Lateral Earth Pressure: At-Rest, Rankine, and Coulomb" (Section 13.10). Additional tables for active earth pressure coefficient based on Mononobe-Okabe's equation have been added.
- In Chapter 14 on "Lateral Earth Pressure: Curved Failure Surface," the passive earth pressure coefficient obtained based on the solution by the lower bound theorem of plasticity and the solution by method of characteristics have been summarized. Also, the section on passive force walls with earthquake forces (Section 14.7) has been expanded.
- In Chapter 15 on "Slope Stability," the parameters required for location of the critical failure circle based on Spencer's analysis have been added.
- Chapter 16 on "Soil Bearing Capacity for Shallow Foundations," includes a new section on continuous foundations under eccentrically-inclined load.
- Chapter 18 is a new chapter titled "An Introduction to Geosynthetics," which examines current developments and challenges within this robust and rapidly expanding area of civil engineering.

In the preparation of an engineering text of this type, it is tempting to include many recent developments relating to the behavior of natural soil deposits found in various parts of the world that are available in journals and conference proceedings with the hope that they will prove to be useful to the students in their future practice. However, based on many years of teaching, the authors feel that clarity in explaining the fundamentals of soil mechanics is more important in a first course in this area than filling the book with too many details and alternatives. Many of the fine points can be left to an advanced course in geotechnical engineering. This approach is most likely to nurture students' interest and appreciation in the geotechnical engineering profession at large.

Trusted Features

Principles of Geotechnical Engineering offers more worked-out problems and figures than any other similar text. Unique in the market, these features offer students ample practice and examples, keeping their learning application-oriented, and helping them prepare for work as practicing civil engineers.

In addition to traditional end-of-chapter exercises, this text provides challenging **critical thinking problems**. These problems encourage deeper analyses and drive students to extend their understanding of the subjects covered within each chapter.

A generous **16-page color insert** features distinctive photographs of rocks and rock-forming minerals. These images capture the unique coloring that help geotechnical engineers distinguish one mineral from another.

Each chapter begins with an introduction and concludes with a summary to help students identify what is most important in each chapter. These features clearly preview and reinforce content to guide students and assist them in retaining key concepts.

A complete, comprehensive discussion addresses the weathering of rocks. Students learn about both weathering and the formation of sedimentary and metamorphic rocks in this thorough presentation.

A detailed explanation focuses on the variation of the maximum and minimum void ratios of granular soils. Students examine variations due to grain size, shape, and non-plastic fine contents.

Resource Materials

A detailed Instructor's Solutions Manual containing solutions to all end-of-chapter problems and Lecture Note PowerPoint Slides are available via a secure, passwordprotected Instructor Resource Center at http://sso.cengage.com.

Principles of Geotechnical Engineering is also available through **MindTap**, Cengage Learning's digital course platform. See the following section on pages xi and xii for more details about this exciting new addition to the book.

Acknowledgments

- We are deeply grateful to Janice Das for her assistance in completing the revision. She has been the driving force behind this textbook since the preparation of the first edition.
- Thanks to Professor Jiliang Li of Purdue University North Central for providing several important review comments on the eighth edition.

The authors would like to thank all of the reviewers and instructors who have provided feedback over the years. In addition we wish to acknowledge and thank our Global Engineering team at Cengage Learning for their dedication to this new book: Timothy Anderson, Product Director; Mona Zeftel, Senior Content Developer; Jana Lewis, Content Project Manager; Kristin Stine, Marketing Manager; Elizabeth Brown and Brittany Burden, Learning Solutions Specialists; Ashley Kaupert, Associate Media Content Developer; Teresa Versaggi and Alexander Sham, Product Assistants; and Rose P. Kernan of RPK Editorial Services. They have skillfully guided every aspect of this text's development and production to successful completion.

> Braja M. Das Khaled Sobhan

Copyright 2018 Cengage Learning. All Rights Reserved. May not be copied, scanned, or duplicated, in whole or in part. WCN 02-200-203

MINDTAP ONLINE COURSE

Principles of Geotechnical Engineering is also available through **MindTap**, Cengage Learning's digital course platform. The carefully-crafted pedagogy and exercises in this market-leading textbook are made even more effective by an interactive, customizable eBook, automatically graded assessments, and a full suite of study tools.

As an instructor using MindTap, you have at your fingertips the full text and a unique set of tools, all in an interface designed to save you time. MindTap makes it easy for instructors to build and customize their course, so you can focus on the most relevant material while also lowering costs for your students. Stay connected and informed through real-time student

tracking that provides the opportunity to adjust your course as needed based on analytics of interactivity and performance. **Algorithmically generated problem sets** allow your students maximum practice while you can be assured that each student is being tested by

 $p = 234$

 Θ kN/m²

unique problems. Videos of real world situations, geotechnical instruments, and soil and rock materials provide students with knowledge of future field experiences.

How does MindTap benefit instructors?

- You can build and personalize your course by integrating your own content into the **MindTap Reader** (like lecture notes or problem sets to download) or pull from sources such as RSS feeds, YouTube videos, websites, and more. Control what content students see with a built-in learning path that can be customized to your syllabus.
- MindTap saves you time by providing you and your students with

Feedback a. The head difference from upstream to downstream side, $H = 6$ m Number of flow channels, $N_f = 5$ Number of equipotential drops, $N_d = 16$ Flow rate beneath the dam, $q = kH \frac{N_f}{N_d}$ $q = (3.7 \times 10^{-6} \text{ m/s})(6 \text{ m})(\frac{5}{16})(24 \times 3600 \text{ s/day}) = 0.5994 \text{ m}^3/\text{m/day}$ b. The head loss between two adjacent equipotential lines = $\frac{H}{N_A} = \frac{6 \text{ m}}{16} = 0.375 \text{ m}$ A piezometer placed at the tip of the sheet pile would have a water level rising to $(6 \text{ m}) - 5(0.375 \text{ m}) = 4.125 \text{ m}$. To three significant figures $h = 4.13 \text{ m}$. c. $p = (10 \text{ m} + 4.125 \text{ m}) \times (9.81 \text{ kN/m}^3) = 139 \text{ kN/m}^2$ Try Another Version

automatically graded assignments and quizzes, including **algorithmically generated problem sets**. These problems include immediate, specific feedback, so students know exactly where they need more practice.

- The **Message Center** helps you to quickly and easily contact students directly from MindTap. Messages are communicated directly to each student via the communication medium (email, social media, or even text message) designated by the student.
- **StudyHub** is a valuable studying tool that allows you to deliver important information and empowers your students to personalize their experience. Instructors can choose to annotate the text with **notes** and **highlights**, share content from the MindTap Reader, and create **flashcards** to help their students focus and succeed.
- The **Progress App** lets you know exactly how your students are doing (and where they might be struggling) with live analytics. You can see overall class engagement and drill down into individual student performance, enabling you to adjust your course to maximize student success.

How does MindTap benefit your students?

- The **MindTap Reader** adds the abilities to have the content read aloud, to print from the reader, and to take notes and highlights while also capturing them within the linked **StudyHub App**.
- The **MindTap Mobile App** keeps students connected with alerts and notifications while also providing them with on-the-go study tools like Flashcards

and quizzing, helping them manage their time efficiently.

- **Flashcards** are pre-populated to provide a jump start on studying, and students and instructors can also create customized cards as they move through the course.
- The **Progress App** allows students to monitor their individual grades, as well as their level compared to the class average. This not only helps them stay on track in the course but also motivates them to do more, and ultimately to do better.
- The unique **StudyHub** is a powerful singledestination studying tool that empowers students to personalize their experience. They can quickly and easily access all notes and highlights marked in the MindTap Reader, locate bookmarked pages, review notes and Flashcards shared by their instructor, and create custom study guides.

To find out more about MindTap go to: www.cengage.com/mindtap.

For more information about MindTap for Engineering, or to schedule a demonstration, please call (800) 354-9706 or email higheredcs@cengage.com. For those instructors outside the United States, please visit http://www.cengage.com/contact/ to locate your regional office.

 \bigodot

cnow
A-Z

^ ●■■ 咚◆■ 野

ABOUT THE AUTHORS

Professor Braja Das is Dean Emeritus of the College of Engineering and Computer Science at California State University, Sacramento. He received his M.S. in Civil Engineering from the University of Iowa and his Ph.D. in the area of Geotechnical Engineering from the University of Wisconsin. He is the author of several geotechnical engineering texts and reference books and has authored more than 300 technical papers in the area of geotechnical engineering. His primary areas of research include shallow foundations, earth anchors, and geosynthetics. He is a Fellow and Life Member of the American Society of Civil Engineers, Life Member of the American Society for Engineering Education, and an Emeritus Member of the Stabilization of Geometrical Materials and Recycled Materials Committee of the Transportation Research Board of the National Research Council (Washington, D.C.). He has previously served as a member of the editorial board of the *Journal of Geotechnical Engineering* of ASCE, a member of the *Lowland Technology International* journal (Japan), associate editor of the *International Journal of Offshore and Polar Engineering* (ISOPE), and co-editor of the *Journal of Geotechnical and Geological Engineering* (Springer, The Netherlands). Presently he is the editor-inchief of the *International Journal of Geotechnical Engineering* (Taylor and Francis, U.K.). Dr. Das has received numerous awards for teaching excellence, including the AMOCO Foundation Award, AT&T Award for Teaching Excellence from the American Society for Engineering Education, the Ralph Teetor Award from the Society of Automotive Engineers, and the Distinguished Achievement Award for Teaching Excellence from the University of Texas at El Paso.

Dr. Khaled Sobhan is a Professor of Civil, Environmental and Geomatics Engineering at Florida Atlantic University. He received his M.S. degree from The Johns Hopkins University, and his Ph.D. degree from Northwestern University, both in the area of Geotechnical Engineering. His primary research areas include ground improvement, geotechnology of soft soils, experimental soil mechanics, and geotechnical aspects of pavement engineering. He served as the Chair of the Chemical and Mechanical Stabilization committee (AFS90) of the Transportation Research Board (2005–2011), and co-authored the TRB Circular titled *Evaluation of Chemical Stabilizers: State-of-the-Practice Report* (E-C086). He is currently serving as an Associate Editor of ASCE's *Journal of Materials in Civil Engineering*, and in the editorial board of the *ASTM Geotechnical Testing Journal, Geotechnical and Geological Engineering* (Springer, The Netherlands), and the *International Journal of Geotechnical Engineering*. He is a recipient of the distinguished *Award for Excellence and Innovation in Undergraduate Teaching* (2006), and the *Excellence in Graduate Mentoring Award* (2009) from Florida Atlantic University. He has authored/co-authored over 100 technical articles and reports in the area of geotechnical engineering.

Copyright 2018 Cengage Learning. All Rights Reserved. May not be copied, scanned, or duplicated, in whole or in part. WCN 02-200-203

CONTENTS

Preface vii MindTap Online Course xi About the Authors xiii

1 Geotechnical Engineering—A Historical Perspective 1

- 1.1 Introduction 1
- 1.2 Geotechnical Engineering Prior to the 18th Century 1
- 1.3 Preclassical Period of Soil Mechanics (1700–1776) 5
- 1.4 Classical Soil Mechanics—Phase I (1776–1856) 6
- 1.5 Classical Soil Mechanics—Phase II (1856–1910) 6
- 1.6 Modern Soil Mechanics (1910–1927) 7
- 1.7 Geotechnical Engineering after 1927 8
- 1.8 End of an Era 13 References 14

2 Origin of Soil and Grain Size 16

- 2.1 Introduction 16
- 2.2 Rock Cycle and the Origin of Soil 16
- 2.3 Rock-Forming Minerals, Rock and Rock Structures 27
- 2.4 Soil-Particle Size 28
- 2.5 Clay Minerals 30
- **2.6** Specific Gravity (G_s) 38
- 2.7 Mechanical Analysis of Soil 39
- 2.8 Particle-Size Distribution Curve 48
- 2.9 Particle Shape 55
- 2.10 Summary 57
	- Problems 57
	- References 63

3 Weight–Volume Relationships 64

- 3.1 Introduction 64
- 3.2 Weight–Volume Relationships 64
- 3.3 Relationships among Unit Weight, Void Ratio, Moisture Content, and Specific Gravity 68
- **3.4** Relationships among Unit Weight, Porosity, and Moisture Content 72

xvi Contents

- 3.5 Relative Density 80
- **3.6** Comments on e_{max} and e_{min} 83
- **3.7** Correlations between e_{max} , e_{min} , $e_{\text{max}} e_{\text{min}}$, and Median Grain Size (D_{50}) 85
- 3.8 Summary 88 Problems 88 References 94

4 Plasticity and Structure of Soil 95

- 4.1 Introduction 95 4.2 Liquid Limit (*LL*) 95
- 4.3 Plastic Limit (*PL*) 105
- 4.4 Plasticity Index 107
- 4.5 Shrinkage Limit (*SL*) 108
- 4.6 Liquidity Index and Consistency Index 113
- 4.7 Activity 114
- 4.8 Plasticity Chart 117
- 4.9 Soil Structure 118

4.10 Summary 123 Problems 124 References 127

5 Classification of Soil 129

- 5.1 Introduction 129
- 5.2 Textural Classification 130
- **5.3** Classification by Engineering Behavior 132
- 5.4 AASHTO Classification System 132
- 5.5 Unified Soil Classification System 136
- **5.6** Comparison between the AASHTO and Unified Systems 139
- 5.7 Summary 150 Problems 151 References 155

6 Soil Compaction 156

- 6.1 Introduction 156
- 6.2 Compaction—General Principles 157
- 6.3 Standard Proctor Test 158
- 6.4 Factors Affecting Compaction 162
- **6.5** Modified Proctor Test 165
- **6.6** Empirical Relationships 167
- **6.7** Structure of Compacted Clay Soil 177
- 6.8 Effect of Compaction on Cohesive Soil Properties 178
- 6.9 Field Compaction 181
- **6.10** Specifications for Field Compaction 186
- **6.11** Determination of Field Unit Weight of Compaction 188
- **6.12** Evaluation of Soils as Compaction Material 195
- 6.13 Special Compaction Techniques 195
- 6.14 Summary 204 Problems 205 References 210

7 Permeability 212

- 7.1 Introduction 212
- 7.2 Bernoulli's Equation 212
- 7.3 Darcy's Law 215
- 7.4 Hydraulic Conductivity 217
- 7.5 Laboratory Determination of Hydraulic Conductivity 218
- 7.6 Relationships for Hydraulic Conductivity—Granular Soil 226
- 7.7 Relationships for Hydraulic Conductivity—Cohesive Soils 232
- 7.8 Directional Variation of Permeability 238
- 7.9 Equivalent Hydraulic Conductivity in Stratified Soil 239
- 7.10 Permeability Test in the Field by Pumping from Wells 244
- 7.11 Permeability Test in Auger Holes 248
- 7.12 Hydraulic Conductivity of Compacted Clayey Soils 250
- 7.13 Moisture Content—Unit Weight Criteria for Clay Liner Construction 252
- 7.14 Summary 253 Problems 254
	- References 259

8 Seepage 261

- 8.1 Introduction 261
- 8.2 Laplace's Equation of Continuity 261
- **8.3** Flow Nets 263
- 8.4 Seepage Calculation from a Flow Net 265
- 8.5 Flow Nets in Anisotropic Soil 271
- 8.6 Mathematical Solution for Seepage 274
- 8.7 Uplift Pressure under Hydraulic Structures 276
- 8.8 Seepage through an Earth Dam on an Impervious Base 277
- 8.9 L. Casagrande's Solution for Seepage through an Earth Dam 280
- 8.10 Pavlovsky's Solution for Seepage through an Earth Dam 282
- 8.11 Filter Design 286
- **8.12 Summary 290** Problems 290 References 294

9 In Situ Stresses 295

- 9.1 Introduction 295
- **9.2** Stresses in Saturated Soil without Seepage 295
- **9.3** Stresses in Saturated Soil with Upward Seepage 301
- **9.4** Stresses in Saturated Soil with Downward Seepage 304
- 9.5 Seepage Force 306
- **9.6** Heaving in Soil Due to Flow around Sheet Piles 309
- **9.7** Use of Filters to Increase the Factor of Safety against Heave 315
- 9.8 Effective Stress in Partially Saturated Soil 318
- 9.9 Capillary Rise in Soils 319
- 9.10 Effective Stress in the Zone of Capillary Rise 322
- 9.11 Summary 325 Problems 325 References 330

10 Stresses in a Soil Mass 331

- 10.1 Introduction 331
- 10.2 Normal and Shear Stresses on a Plane 332
- 10.3 The Pole Method of Finding Stresses along a Plane 336
- 10.4 Stresses Caused by a Point Load 338
- 10.5 Vertical Stress Caused by a Vertical Line Load 341
- 10.6 Vertical Stress Caused by a Horizontal Line Load 343
- 10.7 Vertical Stress Caused by a Vertical Strip Load (Finite Width and Infinite Length) 345
- 10.8 Vertical Stress Caused by a Horizontal Strip Load 350
- 10.9 Linearly Increasing Vertical Loading on an Infinite Strip 354
- 10.10 Vertical Stress Due to Embankment Loading 356
- 10.11 Vertical Stress Below the Center of a Uniformly Loaded Circular Area 360
- 10.12 Vertical Stress at Any Point below a Uniformly Loaded Circular Area 362
- 10.13 Vertical Stress Caused by a Rectangularly Loaded Area 366
- 10.14 Influence Chart for Vertical Pressure 372
- 10.15 Westergaard's Solution for Vertical Stress Due to a Point Load 375
- 10.16 Stress Distribution for Westergaard Material 378
- 10.17 Summary 381 Problems 382 References 389

11 Compressibility of Soil 390

- 11.1 Introduction 390
- 11.2 Contact Pressure and Settlement Profile 391
- 11.3 Relations for Elastic Settlement Calculation 393
- 11.4 Improved Relationship for Elastic Settlement 396
- 11.5 Fundamentals of Consolidation 405
- 11.6 One-Dimensional Laboratory Consolidation Test 409
- 11.7 Void Ratio–Pressure Plots 412
- 11.8 Normally Consolidated and Overconsolidated Clays 415
- 11.9 Effect of Disturbance on Void Ratio–Pressure Relationship 419
- 11.10 Calculation of Settlement from One-Dimensional Primary Consolidation 420
- **11.11** Correlations for Compression Index (C_c) 422
- **11.12** Correlations for Swell Index (C_s) 424
- 11.13 Secondary Consolidation Settlement 431
- 11.14 Time Rate of Consolidation 434
- 11.15 Construction Time Correction of Consolidation Settlement 444
- 11.16 Determination of Coefficient of Consolidation 447
- 11.17 Calculation of Consolidation Settlement under a Foundation 454
- 11.18 Methods for Accelerating Consolidation Settlement 456
- **11.19 Summary 459**
	- Problems 460
		- References 467

12 Shear Strength of Soil 469

- 12.1 Introduction 469
- 12.2 Mohr–Coulomb Failure Criterion 469
- 12.3 Inclination of the Plane of Failure Caused by Shear 471
- 12.4 Laboratory Test for Determination of Shear Strength Parameters 473
- 12.5 Direct Shear Test 473
- 12.6 Drained Direct Shear Test on Saturated Sand and Clay 478
- 12.7 General Comments on Direct Shear Test 481
- 12.8 Triaxial Shear Test-General 486
- 12.9 Consolidated-Drained Triaxial Test 487
- 12.10 Consolidated-Undrained Triaxial Test 497
- 12.11 Unconsolidated-Undrained Triaxial Test 505
- 12.12 Unconfined Compression Test on Saturated Clay 509
- 12.13 Empirical Relationships between Undrained Cohesion (*cu*) and Effective Overburden Pressure (σ_o^{\prime}) 511
- 12.14 Sensitivity and Thixotropy of Clay 512
- 12.15 Strength Anisotropy in Clay 514
- **12.16** Vane Shear Test 516
- 12.17 Other Methods for Determining Undrained Shear Strength 523
- 12.18 Shear Strength of Unsaturated Cohesive Soils 523
- 12.19 Summary 526 Problems 527 References 533

13 Lateral Earth Pressure: At-Rest, Rankine, and Coulomb 535

- 13.1 Introduction 535
- 13.2 At-Rest, Active, and Passive Pressures 535
- 13.3 Earth Pressure At-Rest 538
- 13.4 Earth Pressure At-Rest for Partially Submerged Soil 540
- 13.5 Lateral Pressure on Unyeilding Retaining Walls from Surcharges—Based on Theory of Elasticity 545
- 13.6 Rankine's Theory of Active Pressure 549
- 13.7 Theory of Rankine's Passive Pressure 552
- 13.8 Yielding of Wall of Limited Height 554
- 13.9 Rankine Active and Passive Pressure with Sloping Backfill 555
- 13.10 A Generalized Case for Rankine Active and Passive Pressure—Granular Backfill 558
- 13.11 Diagrams for Lateral Earth-Pressure Distribution against Retaining Walls with Vertical Back 561
- 13.12 Coulomb's Active Pressure 575
- 13.13 Coulomb's Passive Pressure 581
- **13.14** Active Force on Retaining Walls with Earthquake Forces 582
- 13.15 Common Types of Retaining Walls in the Field 594

13.16 Summary 598 Problems 600 References 604

14 Lateral Earth Pressure: Curved Failure Surface 606

- 14.1 Introduction 606
- 14.2 Retaining Walls with Friction 606
- 14.3 Properties of a Logarithmic Spiral 608
- 14.4 Procedure for Determination of Passive Earth Pressure (*Pp*)—Cohesionless Backfill 610
- **14.5** Coefficient of Passive Earth Pressure (K_p) 612
- 14.6 Caquot and Kerisel Solution for Passive Earth Pressure (Granular Backfill) 617
- 14.7 Passive Force on Walls with Earthquake Forces 621
- 14.8 Braced Cuts—General 625
- 14.9 Determination of Active Thrust on Bracing Systems of Open Cuts—Granular Soil 627
- 14.10 Determination of Active Thrust on Bracing Systems for Cuts—Cohesive Soil 629
- 14.11 Pressure Variation for Design of Sheetings, Struts, and Wales 630
- 14.12 Summary 633 Problems 634 References 637

15 Slope Stability 638

- 15.1 Introduction 638
- 15.2 Factor of Safety 640
- 15.3 Stability of Infinite Slopes 641
- 15.4 Infinite Slope with Steady-state Seepage 644
- 15.5 Finite Slopes—General 648
- 15.6 Analysis of Finite Slopes with Plane Failure Surfaces (Culmann's Method) 648
- 15.7 Analysis of Finite Slopes with Circular Failure Surfaces—General 652
- **15.8** Mass Procedure—Slopes in Homogeneous Clay Soil with $\phi = 0$ 653
- **15.9** Slopes in Clay Soil with $\phi = 0$; and c_u Increasing with Depth 662
- **15.10** Mass Procedure—Slopes in Homogeneous $c' \phi'$ Soil 665
- 15.11 Ordinary Method of Slices 671
- 15.12 Bishop's Simplified Method of Slices 680
- 15.13 Stability Analysis by Method of Slices for Steady-State Seepage 682
- **15.14** Solutions for Steady-State Seepage 685
- 15.15 Fluctuation of Factor of Safety of Slopes in Clay Embankment on Saturated Clay 699
- 15.16 Summary 703 Problems 703 References 709

16 Soil Bearing Capacity for Shallow Foundations 710

- 16.1 Introduction 710
- 16.2 Ultimate Soil-Bearing Capacity for Shallow Foundations 712
- 16.3 Terzaghi's Ultimate Bearing Capacity Equation 713
- 16.4 Effect of Groundwater Table 717
- 16.5 Factor of Safety 719
- 16.6 General Bearing Capacity Equation 723
- 16.7 Ultimate Load for Shallow Footings Under Eccentric Load (One-Way Eccentricity) 729
- 16.8 Continuous Footing Under Eccentrically Inclined Load 734
- 16.9 Bearing Capacity of Sand Based on Settlement 740
- 16.10 Summary 742 Problems 742 References 746

17 Subsoil Exploration 748

- 17.1 Introduction 748
- 17.2 Planning for Soil Exploration 749
- 17.3 Boring Methods 750
- 17.4 Common Sampling Methods 754
- 17.5 Sample Disturbance 759
- **17.6** Correlations for $N₆₀$ in Cohesive Soil 760
- 17.7 Correlations for Standard Penetration Number in Granular Soil 761
- 17.8 Other *In Situ* Tests 767
- 17.9 Vane Shear Test 767
- 17.10 Borehole Pressuremeter Test 767
- 17.11 Cone Penetration Test 769
- 17.12 Rock Coring 774
- 17.13 Soil Exploration Report 776
- 17.14 Summary 776 Problems 778 References 781

18 An Introduction to Geosynthetics 783

- 18.1 Introduction 783
- 18.2 Geotextile 784
- 18.3 Geogrid 789
- 18.4 Geomembrane 795
- 18.5 Geonet 799
- 18.6 Geosynthetic Clay Liner 801
- 18.7 Summary 803 References 803

Answers to Selected Problems 805

Index 815

Copyright 2018 Cengage Learning. All Rights Reserved. May not be copied, scanned, or duplicated, in whole or in part. WCN 02-200-203

Geotechnical Engineering— A Historical Perspective

For engineering purposes, *soil* is defined as the uncemented aggregate of mineral grains and decayed organic matter (solid particles) with liquid and gas in the empty spaces between the solid particles. Soil is used as a construction material in various civil engineering projects, and it supports structural foundations. Thus, civil engineers must study the properties of soil, such as its origin, grain-size distribution, ability to drain water, compressibility, strength, and its ability to support structures and resist deformation. *Soil mechanics* is the branch of science that deals with the study of the physical properties of soil and the behavior of soil masses subjected to various types of forces. *Soils engineering* is the application of the principles of soil mechanics to practical problems. *Geotechnical engineering* is the subdiscipline of civil engineering that involves natural materials found close to the surface of the earth. It includes the application of the principles of soil mechanics and rock mechanics to the design of foundations, retaining structures, and earth structures.

1.2 Geotechnical Engineering Prior to the 18th Century

The record of a person's first use of soil as a construction material is lost in antiquity. In true engineering terms, the understanding of geotechnical engineering as it is known today began early in the 18th century (Skempton, 1985). For years, the art of geotechnical engineering was based on only past experiences through a succession

Pyramid/Pharaoh	Location	Reign of Pharaoh
Dioser	Saqqara	$2630 - 2612$ B.C.
Sneferu	Dashur (North)	2612-2589 в.с.
Sneferu	Dashur (South)	2612-2589 в.с.
Sneferu	Meidum	2612-2589 в.с.
Khufu	Giza	$2589 - 2566$ B.C.
Diedefre	Abu Rawash	2566-2558 в.с.
Khafre	Giza	2558-2532 B.C.
Menkaure	Giza	$2532 - 2504$ B.C.

Table 1.1 Major Pyramids in Egypt

of experimentation without any real scientific character. Based on those experimentations, many structures were built—some of which have crumbled, while others are still standing.

Recorded history tells us that ancient civilizations flourished along the banks of rivers, such as the Nile (Egypt), the Tigris and Euphrates (Mesopotamia), the Huang Ho (Yellow River, China), and the Indus (India). Dykes dating back to about 2000 b.c. were built in the basin of the Indus to protect the town of Mohenjo Dara (in what became Pakistan after 1947). During the Chan dynasty in China (1120 b.c. to 249 b.c.) many dykes were built for irrigation purposes. There is no evidence that measures were taken to stabilize the foundations or check erosion caused by floods (Kerisel, 1985). Ancient Greek civilization used isolated pad footings and strip-and-raft foundations for building structures. Beginning around 2700 b.c., several pyramids were built in Egypt, most of which were built as tombs for the country's Pharaohs and their consorts during the Old and Middle Kingdom periods. Table 1.1 lists some of the major pyramids identified through the Pharaoh who ordered it built. As of 2008, a total of 138 pyramids have been discovered in Egypt. Figure 1.1

Figure 1.1 A view of the pyramids at Giza. (*Courtesy of Janice Das, Henderson, Nevada*)

shows a view of the three pyramids at Giza. The construction of the pyramids posed formidable challenges regarding foundations, stability of slopes, and construction of underground chambers. With the arrival of Buddhism in China during the Eastern Han dynasty in 68 A.D., thousands of pagodas were built. Many of these structures were constructed on silt and soft clay layers. In some cases the foundation pressure exceeded the load-bearing capacity of the soil and thereby caused extensive structural damage.

One of the most famous examples of problems related to soil-bearing capacity in the construction of structures prior to the 18th century is the Leaning Tower of Pisa in Italy (See Figure 1.2). Construction of the tower began in 1173 A.D. when the Republic of Pisa was flourishing and continued in various stages for over 200 years. The structure weighs about 15,700 metric tons and is supported by a circular base having a diameter of 20 m (\approx 66 ft). The tower has tilted in the past to the east, north, west, and, finally, to the south. Recent investigations showed that a weak clay layer existed at a depth of about 11 m (\approx 36 ft) below the ground surface compression of which caused the tower to tilt. It became more than 5 m (\approx 16.5 ft) out of plumb

Figure 1.2 Leaning Tower of Pisa, Italy (*Courtesy of Braja M. Das, Henderson, Nevada*)

with the 54 m (\approx 179 ft) height (about a 5.5 degree tilt). The tower was closed in 1990 because it was feared that it would either fall over or collapse. It recently has been stabilized by excavating soil from under the north side of the tower. About 70 metric tons of earth were removed in 41 separate extractions that spanned the width of the tower. As the ground gradually settled to fill the resulting space, the tilt of the tower eased. The tower now leans 5 degrees. The half-degree change is not noticeable, but it makes the structure considerably more stable. Figure 1.3 is an example of a similar problem. The towers shown in Figure 1.3 are located in Bologna, Italy, and they were built in the 12th century. The tower on the left is usually referred to as the *Garisenda Tower*. It is 48 m (\approx 157 ft) in height and weighs about 4210 metric tons. It has tilted about 4 degrees. The tower on the right is the Asinelli Tower, which is 97 m high and weighs 7300 metric tons. It has tilted about 1.3 degrees.

After encountering several foundation-related problems during construction over centuries past, engineers and scientists began to address the properties and

Figure 1.3 Tilting of Garisenda Tower (left) and Asinelli Tower (right) in Bologna, Italy (*Courtesy of Braja M. Das, Henderson, Nevada*)

behaviors of soils in a more methodical manner starting in the early part of the 18th century. Based on the emphasis and the nature of study in the area of geotechnical engineering, the time span extending from 1700 to 1927 can be divided into four major periods (Skempton, 1985):

- **1.** Preclassical (1700 to 1776 A.D.)
- **2.** Classical soil mechanics—Phase I (1776 to 1856 A.D.)
- **3.** Classical soil mechanics—Phase II (1856 to 1910 A.D.)
- **4.** Modern soil mechanics (1910 to 1927 A.D.)

Brief descriptions of some signicant developments during each of these four periods are presented below.

1.3 Preclassical Period of Soil Mechanics (1700–1776)

This period concentrated on studies relating to natural slope and unit weights of various types of soils, as well as the semiempirical earth pressure theories. In 1717, a French royal engineer, Henri Gautier (1660–1737), studied the natural slopes of soils when tipped in a heap for formulating the design procedures of retaining walls. The *natural slope* is what we now refer to as the *angle of repose*. According to this study, the natural slope of *clean dry sand* and *ordinary earth* were 31[°] and 45[°], respectively. Also, the unit weight of clean dry sand and ordinary earth were recommended to be 18.1 kN/m³ (115 lb/ft³) and 13.4 kN/m³ (85 lb/ft³), respectively. No test results on clay were reported. In 1729, Bernard Forest de Belidor (1671–1761) published a textbook for military and civil engineers in France. In the book, he proposed a theory for lateral earth pressure on retaining walls that was a follow-up to Gautier's (1717) original study. He also specified a soil classification system in the manner shown in the following table.

The first laboratory model test results on a 76-mm-high (\approx 3 in.) retaining wall built with sand backfill were reported in 1746 by a French engineer, Francois Gadroy (1705–1759), who observed the existence of slip planes in the soil at failure. Gadroy's study was later summarized by J. J. Mayniel in 1808. Another notable contribution during this period is that by the French engineer Jean Rodolphe Perronet (1708–1794), who studied slope stability around 1769 and distinguished between intact ground and fills.

1.4 Classical Soil Mechanics—Phase I (1776–1856)

During this period, most of the developments in the area of geotechnical engineering came from engineers and scientists in France. In the preclassical period, practically all theoretical considerations used in calculating lateral earth pressure on retaining walls were based on an arbitrarily based failure surface in soil. In his famous paper presented in 1776, French scientist Charles Augustin Coulomb (1736–1806) used the principles of calculus for maxima and minima to determine the true position of the sliding surface in soil behind a retaining wall. In this analysis, Coulomb used the laws of friction and cohesion for solid bodies. In 1790, the distinguished French civil engineer, Gaspard Clair Marie Riche de Prony (1755–1839) included Coulomb's theory in his leading textbook, *Nouvelle Architecture Hydraulique* (Vol. 1). In 1820, special cases of Coulomb's work were studied by French engineer Jacques Frederic Francais (1775–1833) and by French applied mechanics professor Claude Louis Marie Henri Navier (1785–1836). These special cases related to inclined backfills and backfills supporting surcharge. In 1840, Jean Victor Poncelet (1788–1867), an army engineer and professor of mechanics, extended Coulomb's theory by providing a graphical method for determining the magnitude of lateral earth pressure on vertical and inclined retaining walls with arbitrarily broken polygonal ground surfaces. Poncelet was also the first to use the symbol ϕ for soil friction angle. He also provided the first ultimate bearing-capacity theory for shallow foundations. In 1846 Alexandre Collin (1808–1890), an engineer, provided the details for deep slips in clay slopes, cutting, and embankments. Collin theorized that in all cases the failure takes place when the mobilized cohesion exceeds the existing cohesion of the soil. He also observed that the actual failure surfaces could be approximated as arcs of cycloids.

The end of Phase I of the classical soil mechanics period is generally marked by the year (1857) of the first publication by William John Macquorn Rankine (1820–1872), a professor of civil engineering at the University of Glasgow. This study provided a notable theory on earth pressure and equilibrium of earth masses. Rankine's theory is a simplication of Coulomb's theory.

1.5 Classical Soil Mechanics—Phase II (1856–1910)

Several experimental results from laboratory tests on sand appeared in the literature in this phase. One of the earliest and most important publications is one by French engineer Henri Philibert Gaspard Darcy (1803–1858). In 1856, he published a study on the permeability of sand lters. Based on those tests, Darcy dened the term *coef ficient of permeability* (or hydraulic conductivity) of soil, a very useful parameter in geotechnical engineering to this day.

Sir George Howard Darwin (1845–1912), a professor of astronomy, conducted laboratory tests to determine the overturning moment on a hinged wall retaining sand in loose and dense states of compaction. Another noteworthy contribution, which was published in 1885 by Joseph Valentin Boussinesq (1842–1929), was the development of the theory of stress distribution under load bearing areas in a homogeneous, semiinfinite, elastic, and isotropic medium. In 1887, Osborne Reynolds

(1842–1912) demonstrated the phenomenon of dilatancy in sand. Other notable studies during this period are those by John Clibborn (1847–1938) and John Stuart Beresford (1845–1925) relating to the flow of water through sand bed and uplift pressure. Clibborn's study was published in the *Treatise on Civil Engineering, Vol. 2: Irrigation Work in India,* Roorkee, 1901 and also in *Technical Paper No. 97*, Government of India, 1902. Beresford's 1898 study on uplift pressure on the Narora Weir on the Ganges River has been documented in *Technical Paper No. 97*, Government of India, 1902.

1.6 Modern Soil Mechanics (1910–1927)

In this period, results of research conducted on clays were published in which the fundamental properties and parameters of clay were established. The most notable publications are described next.

Around 1908, Albert Mauritz Atterberg (1846–1916), a Swedish chemist and soil scientist, defined *clay-size fractions* as the percentage by weight of particles smaller than 2 microns in size. He realized the important role of clay particles in a soil and the plasticity thereof. In 1911, he explained the consistency of cohesive soils by de fining liquid, plastic, and shrinkage limits. He also defined the plasticity index as the difference between liquid limit and plastic limit (see Atterberg, 1911).

In October 1909, the 17-m (56-ft) high earth dam at Charmes, France, failed. It was built between 1902 and 1906. A French engineer, Jean Fontard (1884–1962), carried out investigations to determine the cause of failure. In that context, he conducted undrained double-shear tests on clay specimens $(0.77 \text{ m}^2 \text{ in area and } 200 \text{ mm})$ thick) under constant vertical stress to determine their shear strength parameters (see Frontard, 1914). The times for failure of these specimens were between 10 to 20 minutes.

Arthur Langley Bell (1874–1956), a civil engineer from England, worked on the design and construction of the outer seawall at Rosyth Dockyard. Based on his work, he developed relationships for lateral pressure and resistance in clay as well as bearing capacity of shallow foundations in clay (see Bell, 1915). He also used shear-box tests to measure the undrained shear strength of undisturbed clay specimens.

Wolmar Fellenius (1876–1957), an engineer from Sweden, developed the stability analysis of undrained saturated clay slopes (that is, $\phi = 0$ condition) with the assumption that the critical surface of sliding is the arc of a circle. These were elaborated upon in his papers published in 1918 and 1926. The paper published in 1926 gave correct numerical solutions for the *stability numbers* of circular slip surfaces passing through the toe of the slope.

Karl Terzaghi (1883–1963) of Austria (Figure 1.4) developed the theory of consolidation for clays as we know today. The theory was developed when Terzaghi was teaching at the American Robert College in Istanbul, Turkey. His study spanned a five-year period from 1919 to 1924. Five different clay soils were used. The liquid limit of those soils ranged between 36 and 67, and the plasticity index was in the range of 18 to 38. The consolidation theory was published in Terzaghi's celebrated book *Erdbaumechanik* in 1925.

Figure 1.4 Karl Terzaghi (1883–1963) (*SSPL via Getty Images*)

1.7 Geotechnical Engineering after 1927

The publication of *Erdbaumechanik auf Bodenphysikalisher Grundlage* by Karl Terzaghi in 1925 gave birth to a new era in the development of soil mechanics. Karl Terzaghi is known as the father of modern soil mechanics, and rightfully so. Terzaghi was born on October 2, 1883 in Prague, which was then the capital of the Austrian province of Bohemia. In 1904 he graduated from the Technische Hochschule in Graz, Austria, with an undergraduate degree in mechanical engineering. After graduation he served one year in the Austrian army. Following his army service, Terzaghi studied one more year, concentrating on geological subjects. In January 1912, he received the degree of Doctor of Technical Sciences from his alma mater in Graz. In 1916, he accepted a teaching position at the Imperial School of Engineers in Istanbul. After the end of World War I, he accepted a lectureship at the American Robert College in Istanbul (1918–1925). There he began his research work on the behavior of soil and settlement of clay and on the failure due to piping in sand under dams. The publication *Erdbaumechanik* is primarily the result of this research.

In 1925, Terzaghi accepted a visiting lectureship at Massachusetts Institute of Technology, where he worked until 1929. During that time, he became recognized as the leader of the new branch of civil engineering called soil mechanics. In October 1929, he returned to Europe to accept a professorship at the Technical University of Vienna, which soon became the nucleus for civil engineers interested in soil mechanics. In 1939, he returned to the United States to become a professor at Harvard University.

The first conference of the International Society of Soil Mechanics and Foundation Engineering (ISSMFE) was held at Harvard University in 1936 with Karl Terzaghi presiding. The conference was possible due to the conviction and efforts of Professor Arthur Casagrande of Harvard University. About 200 individuals representing 21 countries attended this conference. It was through the inspiration and guidance of Terzaghi over the preceding quarter-century that papers were brought to that conference covering a wide range of topics, such as

- Effective stress
- Shear strength
- Testing with Dutch cone penetrometer
- Consolidation
- Centrifuge testing
- Elastic theory and stress distribution
- Preloading for settlement control
- Swelling clays
- Frost action
- Earthquake and soil liquefaction
- Machine vibration
- Arching theory of earth pressure

For the next quarter-century, Terzaghi was the guiding spirit in the development of soil mechanics and geotechnical engineering throughout the world. To that effect, in 1985, Ralph Peck wrote that "few people during Terzaghi's lifetime would have disagreed that he was not only the guiding spirit in soil mechanics, but that he was the clearing house for research and application throughout the world. Within the next few years he would be engaged on projects on every continent save Australia and Antarctica." Peck continued with, "Hence, even today, one can hardly improve on his contemporary assessments of the state of soil mechanics as expressed in his summary papers and presidential addresses." In 1939, Terzaghi delivered the 45th James Forrest Lecture at the Institution of Civil Engineers, London. His lecture was entitled "Soil Mechanics—A New Chapter in Engineering Science." In it, he proclaimed that most of the foundation failures that occurred were no longer "acts of God."

Following are some highlights in the development of soil mechanics and geotechnical engineering that evolved after the first conference of the ISSMFE in 1936:

- Publication of the book *Theoretical Soil Mechanics* by Karl Terzaghi in 1943 (Wiley, New York)
- Publication of the book *Soil Mechanics in Engineering Practice* by Karl Terzaghi and Ralph Peck in 1948 (Wiley, New York)